

PROJECT: Precision Underwater Positioner**ENGINEER: M.Brown****Title: Slope, Deflection, bending stress for Aluminum Wide Flange linkage****Case 1a Concentrated intermediate load; left end free, right end fixed (cantilever)***Roark's Formulas for Stress and Strain 6th edition*

UNITS: kip $\equiv$ 1000·lbf ksi $\equiv \frac{\text{kip}}{\text{in}^2}$ psi $\equiv \frac{\text{lbf}}{\text{in}^2}$ k $\equiv$ kip ksf $\equiv \frac{\text{kip}}{\text{ft}^2}$ psf $\equiv \frac{\text{lbf}}{\text{ft}^2}$ pcf $\equiv \frac{\text{lbf}}{\text{ft}^3}$ plf $\equiv \frac{\text{lbf}}{\text{ft}}$

PROPERTIES & ASSUMPTIONS: Material = Aluminum 6061-T6Modulus of elasticity: $E \equiv 30 \cdot 10^6 \cdot \frac{\text{lbf}}{\text{in}^2}$

WF 2 X 1.43 Wide Flange H-Beam

WF Area moment of inertia: $I \equiv 782 \cdot \text{in}^4$ Load: $W \equiv 45 \cdot \text{lbf}$

WF weight per foot

 $WF := 1.43$ $W = 20.411 \text{ kgf}$ Total Length: $L := 14 \cdot \text{in}$

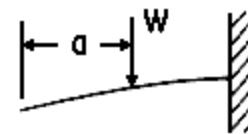
Added weight of beam:

 $W_B := WF \cdot 1.1667$ $L = 0.356 \text{ m}$ $W_B = 1.668 \frac{1}{\text{lb}} \text{ lb}$ Length from end $a := 0 \cdot \text{in}$ **CALCULATION:**

Slope:

$$\theta_A := \frac{W \cdot (L - a)^2}{2 \cdot E \cdot I}$$

$$\theta_A = 1.077 \times 10^{-5} \text{ deg}$$



Deflection:

$$y_A := \frac{-W}{6 \cdot E \cdot I} \cdot (2 \cdot L^3 - 3 \cdot L^2 \cdot a + a^3)$$

$$y_A = -1.754 \times 10^{-6} \text{ in}$$

$$y_A = -4.456 \times 10^{-8} \text{ m}$$

At the right end of the beam (fixed to rotation joint):

Reaction:

$$R_B := W$$

$$R_B = 45 \text{ lbf}$$

$$R_B = 20.411 \text{ kgf}$$

$$R_B = 200.165 \text{ newton}$$

Moment:

$$M_B := -W \cdot (L - a)$$

$$M_B = -630 \text{ lbf} \cdot \text{in}$$

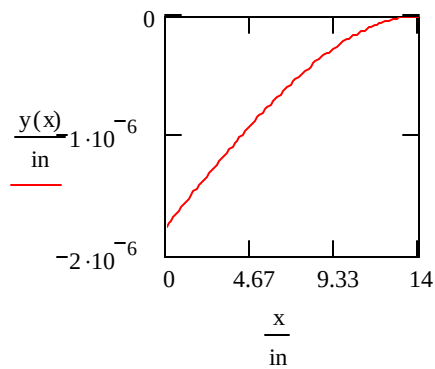
$$M_B = -71.179 \text{ newton} \cdot \text{m}$$

Deflection:

Reaction at end $R_A := 0 \cdot \text{lbf}$ x varies from 0 to end of beam. $x := 0 \cdot L, .01 \cdot L \dots L$

Moment at end $M_A := 0 \cdot \text{lbf}$ A point along the beam length $x_1 := 6 \cdot \text{in}$

$$y(x) := y_A + \theta_A \cdot x + \frac{M_A \cdot x^2}{2 \cdot E \cdot I} + \frac{R_A \cdot x^3}{6 \cdot E \cdot I} - (x > a) \cdot \left[\frac{W}{6 \cdot E \cdot I} \cdot (x - a)^3 \right]$$

**Stress due to Bending**

section modulus $S := .481 \cdot \text{in}^3$

$$\sigma_B := \frac{M_B}{S} \quad \sigma_B = -1.31 \times 10^3 \text{ psi}$$

$$\sigma_B = -9.031 \times 10^6 \text{ Pa}$$