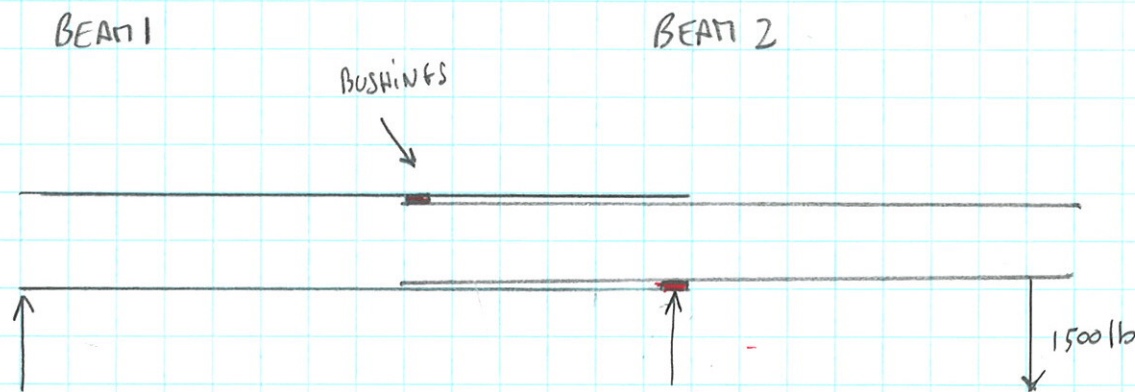


BEAM CALCULATIONS FOR PARAGON CTD BOOM

12/3/2015
FRANCOIS CAZENAVE



BEAM 1 : $3 \times 3 \times \frac{3}{16} \times 42$

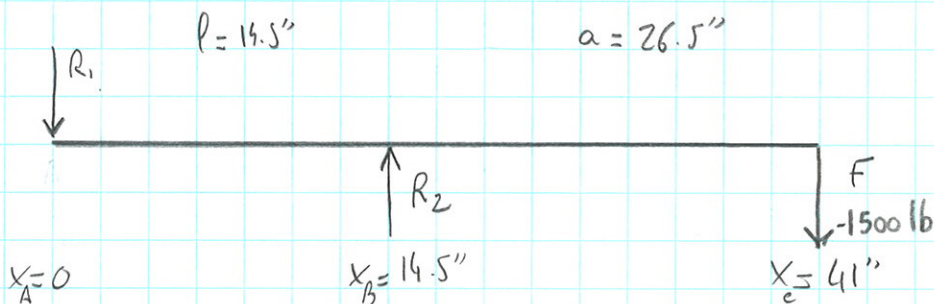
MATERIAL: A36 STEEL , $E = 29 \times 10^6$ PSI
 $\gamma_s = 38000$ PSI

MOMENT OF INERTIA : $I = 2.6 \text{ in}^4$
SECTION MODULUS : $S = 1.73 \text{ in}^3$

BEAM 2 : $2.5 \times 2.5 \times \frac{1}{4} \times 44$

$I = 1.69 \text{ in}^4$
 $S = 1.35 \text{ in}^3$

CONSIDERING BEAM 2 . WE ASSUME THE BEAMS DO NOT TOUCH EACH OTHER EXCEPT AT BUSHINGS



USING EQUATIONS
AND NOTATIONS FROM
SHIGLEY'S PAGE 1017

$$R_1 = F \times a / p = -2741 \text{ lb}$$

$$R_2 = \frac{F}{p} (p + a) = \frac{1500 \times 41}{14.5} = +4241 \text{ lb}$$

SHEAR FORCE

$$V_{AB} = -2741 \text{ lb}$$

$$V_{BC} = -1500 \text{ lb}$$

MOMENT

$$M_{AB} = 2741 \times x$$

$$\begin{aligned} M_{BC} &= -1500 \times (x - l - a) \\ &= -1500 x + 61500 \end{aligned}$$

DISPLACEMENT:

AT END OF BEAM

$$y_c = - \frac{F a^2}{3EI} (l + a) = \frac{1500 \times 26.5^2}{3 \times 29 \times 10^6 \times 1.63} \times 41 = 0.29''$$

AT $x=7''$ (WHERE IT IS MOST LIKELY TO INTERFERE WITH OTHER BEAM)

$$\begin{aligned} y &= \frac{F a x x}{6 E I l} (l^2 - x^2) = \frac{1500 \times 26.5 \times 7}{6 \times 29 \times 10^6 \times 1.63 \times 14.5} (14.5^2 - 7^2) \\ &= 0.01'' \ll 1/16 \text{ GAP} \end{aligned}$$

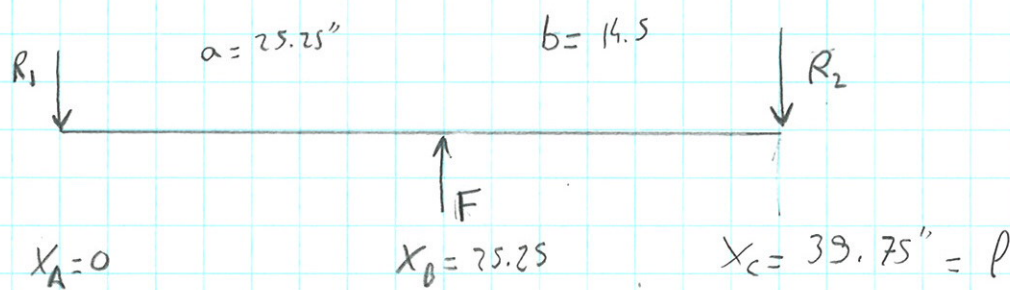
THERE IS NO INTERFERENCE EXCEPT AT BUSHINGS

MAX STRESS IN BEAM (USING EQUATION 3-26B FROM SHIGLEY, PAGE 50)

$$\sigma_{\text{MAX}} = \frac{M_B}{S} = \frac{2741 \times 14.5}{1.35} = 29440 \text{ PSI}$$

RESULT MATCHES FEA

NOW WE CONSIDER BEAM 1, USING RESULTING FORCE
FROM BEAM 2 CALCULATIONS



ASSUMING SIMPLE
SUPPORTS
EQUATIONS FROM
SHIGLEY'S PAGE 1015

$$F = 4241 \text{ lb} \quad (\text{FROM CALCULATIONS ON BEAM 2})$$

MAXIMUM MOMENT IS AT X_B

$$M_B = \frac{F b a}{L} = \frac{4241 \times 14.5 \times 25.25}{39.75} = 39062 \text{ lb.in}$$

MAXIMUM STRESS

$$\sigma_{\max} = \frac{M_B}{S} = \frac{39062}{1.73} = 22579 \text{ PSI}$$

RESULT MATCHES FEA

CONCLUSION: STRESS IN BEAM 2 IS CLOSE TO σ_s

STRESS IN BEAM 1 IS MUCH LOWER THAN σ_s

BEARINGS MUST BE DESIGNED TO HANDLE RESULTING
FORCES

ASSUMING LOAD IS APPLIED AT 45° FROM VERTICAL, TOWARD BACK OR FRONT OF BOAT

MOMENT OF INERTIA DOES NOT CHANGE BECAUSE IT'S A SQUARE SECTION

SECTION MODULUS CHANGES:

$$S = \frac{I}{c} \quad (c \text{ IS MAX DISTANCE FROM CENTER})$$

$$\text{BEAM 1: } - S = 2.6 / 1.366 = 1.32 \text{ in}^3$$

$$\text{BEAM 2} \quad S = 1.63 / 1.56 = 1.08 \text{ in}^3$$

C CALCULATED
IN SOLIDWORKS
SKETCHES

MAX STRESS IN BEAM 2:

$$\sigma_{\text{MAX}} = \frac{M_B}{S} = \frac{2741 \times 14.5}{1.08} = 36800 \text{ PSI}$$

FROM PAGE 2

RESULT

MAX STRESS IN BEAM 1:

$$\sigma_{\text{MAX}} = \frac{M_B}{S} = \frac{39062}{1.32} = 29592 \text{ PSI}$$

FROM PAGE 3

CONCLUSION: STRESS IN BEAM 2 IS AT YS AT 45° LOAD

POSSIBLE SOLUTIONS: - INCREASE WALL THICKNESS OF BEAM 2 TO 5/16"

- REDUCE MAXIMUM DESIGN LOAD AT 45°

- DETERMINE THAT YS IS GREATER THAN 36800 PSI